

# Nonlinear Vibration Control of Piezoelectric-Elastic-Piezoelectric Sandwich Beam

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## Abstract

This work presents the active control of nonlinear vibrations of a piezoelectric-elastic-piezoelectric sandwich beam, subjected to transverse excitation while neglecting axial displacement effects. By using a structure with piezoelectric actuators and sensors, and taking into account geometric nonlinearities, a nonlinear vibration control model was obtained through a feedback control law, which is a proportional-derivative controller. The dynamic equation of the structure is derived by applying the variational principle and Hamilton's principle. This equation is solved under primary and secondary resonance by adopting the method of averaging as a perturbation scheme and Galerkin's approximation. The simulation results of amplitude-frequency responses are presented and discussed for different values of control gains and for three boundary conditions. Our results are in good agreement with those obtained by other methods.

## Keywords

Piezoelectric, Sandwich Beam, Nonlinear Vibrations, Active Control, Method of Averaging

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## 1. Introduction

Structural vibrations are highly undesirable, as they can lead to issues such as structural fatigue, transmission of vibrations to other systems, and external or internal noise due to acoustic radiation, among others [1]-[3]. In many industrial and defense applications, noise and vibrations represent a major challenge. Conventional vibration attenuation methods based on passive devices, such as tuned

mass dampers or viscoelastic treatments, exhibit limited efficiency at low frequencies. At such frequencies, the kinetic energy involved is relatively small, which requires very large auxiliary masses to achieve noticeable effects, often making the solution impractical. In addition, the intrinsic dissipation of damping materials decreases when the strain rate is low, thereby reducing their ability to absorb vibratory energy. Finally, passive devices have a narrow bandwidth and are highly sensitive to mistuning: even slight shifts in the natural frequency of the structure, caused, for instance, by temperature or load variations, can significantly degrade their performance. These limitations explain why passive control strategies are generally ineffective at low frequencies and highlight the relevance of active control methods [4]-[9]. The principle of these so-called active techniques is to generate a field that interferes with the disturbance field. The superimposed field must therefore match the disturbance in amplitude but be opposite in phase for each relevant frequency. While the principle is straightforward, its implementation is much more complex, as the disturbance is often unpredictable and composed of multiple frequencies. Moreover, disturbance minimization is often required over a wide spatial domain, further complicating the problem [10].

Although active control was conceived in the 1930s, it only truly advanced with the emergence of digital signal processors in the 1980s. While some applications of this technology have already been developed, many are still under research, particularly in the aerospace, avionics, and automotive sectors. Focusing specifically on active vibration control, advancements remain relatively recent. In fact, the additional size and mass introduced by the sensors and actuators required for active vibration control have long hindered the development of many applications. Only in the last few decades has the use of piezoelectric material-based transducers enabled significant progress. Due to their compactness, low weight, and electromechanical conversion capabilities, piezoelectric materials exhibit all the necessary qualities for use in active vibration control systems. Moreover, they can serve both as electromechanical actuators and vibration sensors in the system [11] [12].

To reduce stresses in materials, extend their service life, enhance structural safety (e.g., in transportation), and improve user comfort, the control and damping of mechanical vibrations have been the subject of extensive scientific research over many decades. Furthermore, the recent proliferation of so-called “smart materials”, which couple multiple physical fields such as mechanics and electricity, has led to the development of reliable, robust, and efficient vibration control techniques that are also highly integrable. These techniques are therefore well-suited for embedded systems or structures with strict space constraints [11]. In this regard, Rechdaoui *et al.* [12]-[14] developed an active control method for nonlinear vibrations of a piezoelectric-elastic sandwich beam based on the method of multiple scales. Similarly, Belouettar *et al.* [15] proposed an active control approach for the same structure based on the harmonic balance method. Despite such contributions, vibration-related damage remains a persistent problem in our socie-

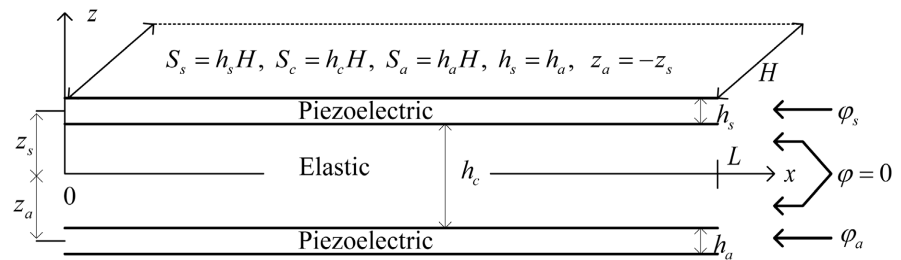
ties, and many avenues still remain to be explored; hence the ongoing need for research efforts.

This work aims to contribute to the active control of nonlinear vibrations of a sandwich beam using the method of averaging [16]-[20].

## 2. Mathematical Modeling

### 2.1. Theoretical Formulation

The beam under investigation consists of an elastic core sandwiched between two piezoelectric face sheets polarized through their thickness, as shown in **Figure 1**. Euler-Bernoulli beam theory is applied to the face sheets, which are assumed to resist membrane and bending stresses. Timoshenko beam theory is adopted for the core, which is assumed to also resist transverse shear stress. The piezoelectric layers are fully covered on their top and bottom surfaces with electrodes. The elastic and piezoelectric materials are considered orthotropic, with their orthotropy axes aligned with those of the sandwich beam. All layers are assumed to be perfectly bonded. The transverse normal stress is considered negligible compared to the other stress components [21]-[24]. The length, width, and thickness of the beam are denoted by  $L$ ,  $H$ , and  $h$ , respectively. The subscripts  $a$ ,  $s$ , and  $c$  refer to the quantities associated with the bottom and top face sheets and the core, respectively.



**Figure 1.** Piezoelectric-elastic-piezoelectric sandwich beam [14].

### 2.2. Kinematic Description of the Beam

According to the classical laminate theory based on the aforementioned assumptions, the displacement fields are described in [1] [11] [13] [24].

$$\begin{cases} u(x, z, t) = u(x, t) - zw_{,x}(x, t) \\ v(x, z, t) = 0 \\ w(x, z, t) = w(x, t) \end{cases} \quad (1)$$

In the theory of beams undergoing large deformations, the Green-Lagrange strain tensor is considered without linearization and is simplified according to the Von Kármán assumptions.

$u$  : longitudinal displacement;

$w$  : transverse displacement;

$z$  : coordinate along the beam thickness (thickness direction).

Thus, the strain in the x-direction is given by:

$$\begin{cases} \varepsilon = \varepsilon_0 - zW_{,xx} \\ \varepsilon_0 = u_{,x} + \frac{1}{2}w_{,x}^2 \end{cases} \tag{2}$$

### 2.3. Electromechanical Coupling

It is well known that in piezoelectric materials, the electric field and strain mutually influence each other. This property enables the use of piezoelectric materials as sensors and actuators for vibration control. More specifically, this relationship can be described by constitutive equations that characterize the coupling effects between mechanical and electrical properties [1] [24].

$$\begin{cases} \sigma = c\varepsilon - e^t E \\ D = e\varepsilon + \epsilon E \end{cases} \tag{3}$$

- $\sigma$  : Stress tensor;
- $\varepsilon$  : Strain tensor;
- $D$  : Electric displacement vector;
- $E$  : Electric field vector;
- $c$  : Elasticity matrix;
- $e$  : Piezoelectric matrix;
- $\epsilon$  : Dielectric permittivity.

The constitutive equations of piezoelectric materials can be written in the following expanded form:

$$\begin{pmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \\ \sigma_4 \\ \sigma_5 \\ \sigma_6 \\ D_1 \\ D_2 \\ D_3 \end{pmatrix} = \begin{bmatrix} c_{11} & c_{12} & c_{13} & 0 & 0 & 0 \\ c_{21} & c_{22} & c_{23} & 0 & 0 & 0 \\ c_{31} & c_{32} & c_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & c_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & c_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & c_{66} \end{bmatrix} \begin{pmatrix} 0 & 0 & -e_{13} \\ 0 & 0 & -e_{13} \\ 0 & 0 & -e_{13} \\ 0 & -e_{13} & 0 \\ -e_{13} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \begin{pmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \varepsilon_3 \\ \varepsilon_4 \\ \varepsilon_5 \\ \varepsilon_6 \end{pmatrix} + \begin{bmatrix} 0 & 0 & 0 & 0 & e_{15} & 0 \\ 0 & 0 & 0 & e_{24} & 0 & 0 \\ e_{31} & e_{32} & e_{33} & 0 & 0 & 0 \end{bmatrix} \begin{pmatrix} \epsilon_{11} & 0 & 0 \\ 0 & \epsilon_{22} & 0 \\ 0 & 0 & \epsilon_{33} \end{pmatrix} \begin{pmatrix} E_1 \\ E_2 \\ E_3 \end{pmatrix} \tag{4}$$

Displacements are considered independent of  $y$  and zero along the  $y$ -direction; the stress tensor is uniaxial, and the directions of the vectors  $D$  and  $E$  are parallel to the  $z$ -axis. Thus, the reduced constitutive relations are given by:

$$\begin{cases} \varepsilon_2 = 0 \\ \sigma_3 = 0 \end{cases} \Rightarrow \varepsilon_3 = \frac{1}{c_{33}}(e_{33}E_3 - c_{13}\varepsilon) \tag{5}$$

It follows that:

$$\begin{pmatrix} \sigma_1 \\ D_3 \end{pmatrix} = \begin{bmatrix} c_{11}^* & -e_{31}^* \\ e_{31}^* & \epsilon_{33}^* \end{bmatrix} \begin{pmatrix} \varepsilon \\ E_3 \end{pmatrix} \tag{6}$$

With:  $\epsilon_{33}^* = \epsilon_{33} + \frac{e_{33}^2}{c_{33}}$ ;  $e_{31}^* = e_{31} - \frac{c_{13}}{c_{33}}e_{33}$ ;  $c_{11}^* = c_{11} - \frac{c_{13}^2}{c_{33}}$ .

## 2.4. Feedback Control Law

Let us now consider an arbitrary piezoelectric layer, actuator, or sensor, placed between  $z_-, z_+$  ( $z_- < z_+$ ), with center  $z_c = (z_- + z_+)/2$  and thickness  $h$ . The electrostatic equilibrium equation, assuming no volume charge density, is given by:

$$\frac{\partial D_3}{\partial z} = 0 \quad (7)$$

Using the boundary conditions,  $D(z_-) = 0$  or  $D(z_+) = 0$ , we have:

$$D_3(z) = 0 \quad (8)$$

Thus, the electric field in the sensor, as a function of displacement, is given by:

$$E_3(z) = -\frac{e_{31}^*}{\epsilon_{33}^*} \varepsilon = -\frac{e_{31}^*}{\epsilon_{33}^*} \left( u_{,x} + \frac{1}{2} w_{,x}^2 - z w_{,xx} \right) \quad (9)$$

Since the electric field is derived from a potential, we have:

$$E_3 = -\frac{\partial \phi}{\partial z} \quad (10)$$

Consequently, the potential difference is given by:

$$\Delta \phi = \phi(z_+) - \phi(z_-) = -\int_{z_-}^{z_+} E_3(z) dz = \frac{e_{31}^*}{\epsilon_{33}^*} h_i \left( u_{,x} + \frac{1}{2} w_{,x}^2 - z_i w_{,xx} \right) \quad (11)$$

With:  $z_- = z_i - \frac{h_i}{2}$  and  $z_+ = z_i + \frac{h_i}{2} \Big|_{i=a,s}$ .

From Equations (9) and (11), we have:

$$E_3(z) = -\frac{\Delta \phi}{h_i} + \frac{e_{31}^*}{\epsilon_{33}^*} (z - z_i) w_{,xx} \quad (12)$$

The core of the beam is assumed to be conductive with a uniform potential set to zero. The sensor potential, denoted  $\phi_s(x)$ , is then given by:

$$\phi_s = \Delta \phi_s = \frac{e_{31}^*}{\epsilon_{33}^*} h_s \left( u_{,x} + \frac{1}{2} w_{,x}^2 - z_s w_{,xx} \right) \quad (13)$$

The actuator potential  $\phi_a(x)$  depends on the sensor output potential  $\phi_s(x)$  through a proportional-derivative control law described as follows:

$$\phi_a = G_p \phi_s + G_d \dot{\phi}_s \quad (14)$$

In this formulation,  $G_p$  represents the proportional gain, which acts directly on the sensor displacement and thus modifies the effective stiffness perceived by the structure. By adjusting  $G_p$ , the stiffening or softening behavior of the system can be influenced, and the effective natural frequency can be shifted.  $G_d$  on the other hand, is the derivative (or velocity) gain, which acts on the sensor velocity and contributes to the effective damping of the system, reducing vibration amplitudes and the hysteresis associated with nonlinear responses.

Using Equations (12) and (13), the electric fields in the sensor and actuator are,

respectively, given by:

$$E_3^s(z) = -\frac{\phi_s}{h_s} + \frac{e_{31}^*}{\epsilon_{33}^*}(z - z_s)w_{,xx} \tag{15}$$

$$E_3^a(z) = \frac{\phi_a}{h_a} + \frac{e_{31}^*}{\epsilon_{33}^*}(z - z_a)w_{,xx} \tag{16}$$

The potentials  $\phi_a$  and  $\phi_s$  are independent of  $z$  and  $z_s = (h_c + h_s)/2$  et  $z_a = -(h_c + h_s)/2$ .

The direct and inverse piezoelectric coefficients were taken into account in these formulations and will be involved in the dynamic behavior of the beam [1] [12] [25] [26].

### 2.5. Dynamic Equation

To determine the dynamic equation of the beam, we use the variational formulation, Hamilton's principle, and Equations (5), (14), and (15). The beam is subjected to axial and transverse excitations  $F_x$  and  $F_z$  [1] [24] [27] [28].

For the variational principle, we have:

$$\begin{aligned} \int_V \sigma_1 \delta \epsilon dv &= \int_{V_s} \sigma_1 \delta \epsilon dv_s + \int_{V_c} \sigma_1 \delta \epsilon dv_c + \int_{V_a} \sigma_1 \delta \epsilon dv_a \\ &= \int_0^L (N \delta \epsilon_0 + M \delta w_{,xx}) dx \\ &= \int_0^L (F_x \delta u + F_z \delta w) dx - (\rho S)_* \int_0^L (\ddot{u} \delta u + \ddot{w} \delta w) dx \end{aligned} \tag{17}$$

With: 
$$\begin{cases} (\rho S)_* = \rho_s S_s + \rho_c S_c + \rho_a S_a \\ \delta \epsilon_0 = \delta u_{,x} + w_{,x} \delta w_{,x} \end{cases}$$

According to Hamilton's principle, we have:

$$\begin{cases} N_{,xx} = \int_s \sigma_{xx} ds = ES \left[ \frac{\partial u}{\partial x} + \frac{1}{2} \left( \frac{\partial w}{\partial x} \right)^2 \right] \\ M_{,xx} = \int_s z \sigma_{xx} ds = -EI \frac{\partial^2 w}{\partial x^2} \end{cases} \tag{18}$$

If we integrate over the entire thickness and width, and assume that the piezoelectric layers are symmetrical ( $h_a = h_s$ ), the axial force  $N$  and the bending moment  $M$  are determined from the previous equation as follows:

$$N = \sigma_1^a S_a + \sigma_1^c S_c + \sigma_1^s S_s = (\sigma_1^a + \sigma_1^s) S_s + E_c S_c \epsilon \tag{19}$$

Using Equation (5), we obtain:

$$\sigma_1^a + \sigma_1^s = 2c_{11}^* \epsilon - e_{31}^* (E_3^a + E_3^s) \tag{20}$$

$$E_3^a + E_3^s = \frac{\phi_a - \phi_s}{h_s} \tag{21}$$

Using Equations (2), (13)-(16), we obtain:

$$N = \left( 2c_{11}^* S_s + E_c S_c + (1 - G_p) S_s \frac{(e_{31}^*)^2}{\epsilon_{33}^*} \right) \epsilon_0 - (1 - G_p) S_s \frac{(e_{31}^*)^2}{\epsilon_{33}^*} z_s w_{,xx} - G_d S_s \frac{(e_{31}^*)^2}{\epsilon_{33}^*} (\dot{u}_{,x} + w_{,x} \dot{w}_x - z_s w_{,xx}) \tag{22}$$

We also obtain the moment in the same manner; thus, we have:

$$\begin{cases} N = (ES)_* \epsilon_0 - B_N w_{,xx} - (ES)_{pe} G_d (\dot{u}_{,x} + w_{,x} \dot{w}_x - \dot{w}_{,xx} z_s) \\ M = -B_M \epsilon_0 + (EI)_* w_{,xx} - (ES)_{pe} z_s G_d (\dot{u}_{,x} + w_{,x} \dot{w}_x - \dot{w}_{,xx} z_s) \end{cases} \tag{23}$$

$$\text{With: } \begin{cases} (ES)_* = E_c S_c + 2c_{11}^* S_s + (ES)_{pe} (1 - G_p); \\ (ES)_{pe} = S_s \frac{(e_{31}^*)^2}{\epsilon_{33}^*}; \\ B_N = (ES)_{pe} (1 - G_p) z_s; \\ B_M = (ES)_{pe} (1 + G_p) z_s; \\ (EI)_* = E_c I_c + 2c_{11}^* (I_s + S_s z_s^2) + \frac{(ES)_{pe}}{S_s} (2I_s + (1 + G_p) z_s^2 S_s). \end{cases}$$

By applying the variational principle to the displacements  $u$  and  $w$ , and integrating by parts once for the terms in  $\delta u_{,x}$  and  $\delta w_{,x}$ , and twice for the terms in  $\delta w_{,xx}$ , we obtain the following partial differential equations [1] [12]-[15].

$$\begin{cases} -N_{,x} + (\rho S)_* \ddot{u} = F_X \\ M_{,xx} - (Nw_{,x})_{,x} + (\rho S)_* \ddot{w} = F_Z \end{cases} \tag{24}$$

Assuming that the axial force and the axial displacement inertia are negligible, system (24) becomes:

$$\begin{cases} -N_{,x} = 0 \\ M_{,xx} - (Nw_{,x})_{,x} + (\rho S)_* \ddot{w} = F_Z \end{cases} \tag{25}$$

The axial force depends only on time ( $N(x,t) = N(t)$ ), so system (25) becomes:

$$\begin{cases} N = \frac{1}{2} (ES)_* w_{,x}^2 - B_N w_{,xx} - (ES)_{pe} G_d (w_{,x} \dot{w}_x - \dot{w}_{,xx} z_s) \\ M = \frac{1}{2} B_M w_{,x}^2 + (EI)_* w_{,xx} - (ES)_{pe} G_d z_s (w_{,x} \dot{w}_x - \dot{w}_{,xx} z_s) \end{cases} \tag{26}$$

En intégrant la première équation du système (26) entre (O et L), on obtient:

$$N(t) = \frac{1}{2L} (ES)_* \int_0^L w_{,x}^2 dx - \frac{B_N}{L} \int_0^L w_{,xx} dx - \frac{(ES)_{pe}}{L} G_d \int_0^L (w_{,x} \dot{w}_x - \dot{w}_{,xx} z_s) dx \tag{27}$$

By substituting (27) into the second equation of system (26), the following dynamic equation is obtained:

$$\begin{aligned} & (\rho S)_* \ddot{w} + (EI)_* w_{,xxxx} - N(t) w_{,xx} - B_M (w_{,xx}^2 + w_{,x} w_{,xxx}) \\ & - (\rho S)_{pe} G_d z_s (\dot{w}_{,x} w_{,xxx} + 2\dot{w}_{,xx} w_{,xx} + w_{,x} \dot{w}_{,xxx} - z_s \dot{w}_{,xxxx}) = F_Z \end{aligned} \tag{28}$$

This differential equation describes the transverse dynamic behavior of the piezoelectric-elastic-piezoelectric beam subjected to a transverse excitation and active control based on the feedback control law when the axial force and axial displacement effects are neglected. The free and forced nonlinear vibrations, as well as the active control of the beam, can be analyzed by solving Equation (25) or (28) [12].

In this work, the axial effects are neglected; therefore, only Equation (28), which describes the dynamic behavior of the beam without axial effects, is solved.

### 3. Solution Methodology

To solve Equation (28), and in order to simplify the calculations, the Galerkin approximation given by Equation (30) below is applied. The beam is transversely excited by an external, uniformly distributed harmonic force of the form:

$$F_z(x, t) = f(x) \cos(\omega t) \tag{29}$$

The Galerkin approximation [12] [28] is given as follows:

$$w(x, t) = \sum_{k=1}^n q_k(t) \varphi_k(x) \tag{30}$$

$\varphi_k(x)$ : are the vibration modes of the beam;

$q_k(t)$ : are the associated time-dependent amplitudes.

This mode superposition leads to a reduced-order approximate dynamic system model. To perform control in a simple manner, we consider a single mode. By substituting Equation (30) into Equation (28), integrating over the entire length, and omitting the indices since only one mode is considered, we obtain:

$$\ddot{q}(t) + 2\mu\dot{q}(t) + \omega_L^2 q(t) + \alpha_2 q^2(t) + \alpha_3 q^3(t) + \alpha_4 q(t)\dot{q}(t) + \alpha_5 q^2(t)\dot{q}(t) = F_1 \cos(\omega t) \tag{31}$$

With:

$$2\mu = \frac{(ES)_{pe} G_d z_s^2}{M} \int_0^L \varphi_{xxxx}(x) \varphi(x) dx ;$$

$$\omega_L^2 = \frac{(EI)_*}{M} \int_0^L \varphi_{xxxx}(x) \varphi(x) dx ;$$

$$\alpha_2 = \frac{B_N}{ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L \varphi_{xx}(x) dx - \frac{B_M}{M} \int_0^L \left\{ (\varphi_{xx}(x))^2 + \varphi_x(x) \varphi_{xxx}(x) \right\} \varphi(x) dx ;$$

$$\alpha_3 = -\frac{(ES)_*}{2ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L (\varphi_x(x))^2 dx ;$$

$$\alpha_4 = -\frac{(ES)_{pe} G_d z_s}{M} \left( \frac{1}{L} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L \varphi_{xx}(x) dx + 2 \left( \int_0^L (\varphi_{xx}(x))^2 + \varphi_x(x) \varphi_{xxx}(x) \varphi(x) dx \right) \right) ;$$

$$\alpha_5 = \frac{(ES)_{pe} G_d}{ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L (\varphi_x(x))^2 dx ;$$

$$F_1 = \frac{1}{M} \int_0^L f(x) \varphi(x) dx ;$$

$$M = (\rho S)_* \int_0^L (\varphi_x(x))^2 dx .$$

These coefficients depend on the control parameters  $G_p$  and  $G_{db}$  and consequently, they can be significantly influenced by the control law considered. The resolution of Equation (31) will be carried out in the following, using the method of averaging.

### 3.1. Primary Resonance

According to the principle of the method of averaging, Equation (30) can be re-written by introducing the perturbation parameter; thus, we have:

$$\ddot{q} + \omega_L^2 q = -\varepsilon \left[ 2\mu\dot{q} + \alpha_2 q^2 + \alpha_3 q^3 + \alpha_4 q\dot{q} + \alpha_5 q^2 \dot{q} - F_1 \cos(\omega t) \right] \quad (32)$$

If  $\varepsilon = 0$ , the general solution of Equation (31) is given by:

$$q = a \cos(\omega_L t + \beta) \quad (33)$$

Since  $a$  and  $\beta$  are constants, the derivative of Equation (32) is:

$$\dot{q} = -a\omega_L \sin(\omega_L t + \beta) \quad (34)$$

If  $\varepsilon \neq 0$ , the solution of Equation (32) takes the form of Equation (34), but with  $a$  and  $\beta$  now varying with time. Differentiating Equation (33) then yields:

$$\dot{q} = -a\omega_L \sin(\omega_L t + \beta) + \dot{a} \cos(\omega_L t + \beta) - a\dot{\beta} \sin(\omega_L t + \beta) \quad (35)$$

By comparing Equations (34) and (35), we obtain:

$$\dot{a} \cos(\omega_L t + \beta) - a\dot{\beta} \sin(\omega_L t + \beta) = 0 \quad (36)$$

Let us differentiate Equation (33) with respect to time:

$$\ddot{q} = -\dot{a}\omega_L \sin(\omega_L t + \beta) - a\omega_L^2 \cos(\omega_L t + \beta) - a\dot{\beta}\omega_L \cos(\omega_L t + \beta) \quad (37)$$

By substituting (34), (35), and (37) into (32), we obtain:

$$\begin{aligned} & \dot{a}\omega_L \sin(\omega_L t + \beta) + a\dot{\beta}\omega_L \cos(\omega_L t + \beta) \\ &= -2\mu\varepsilon(a\omega_L \sin(\omega_L t + \beta)) + \alpha_2\varepsilon(a^2 \cos^2(\omega_L t + \beta)) \\ & \quad + \alpha_3\varepsilon(a^3 \cos^3(\omega_L t + \beta)) - \alpha_4\varepsilon(a^2 \omega_L \cos(\omega_L t + \beta) \sin(\omega_L t + \beta)) \\ & \quad - \alpha_5\varepsilon(a^3 \omega_L \cos^2(\omega_L t + \beta) \sin(\omega_L t + \beta)) - \varepsilon F_1 \cos(\omega t) \end{aligned} \quad (38)$$

By using (36) and (38), we have:

$$\begin{aligned} \dot{a} = \frac{\varepsilon}{\omega_L} & \left\{ -\mu a \omega_L + \mu a \omega_L \cos(2\omega_L t + 2\beta) + \alpha_2 \left( \frac{1}{4} a^2 \sin(\omega_L t + \beta) + \frac{1}{4} a^2 \sin(3\omega_L t + 3\beta) \right) \right. \\ & + \alpha_3 \left( \frac{1}{4} a^3 \sin(2\omega_L t + 2\beta) + \frac{1}{8} a^3 \sin(4\omega_L t + 4\beta) \right) - \frac{1}{2} F_1 \sin((\omega_L - \omega)t + \beta) \\ & - \alpha_4 \left( \frac{1}{4} a^2 \omega_L \cos(\omega_L t + \beta) - \frac{1}{4} a^2 \omega_L \cos(3\omega_L t + 3\beta) \right) - \frac{1}{2} F_1 \sin((\omega_L + \omega)t + \beta) \\ & \left. - \alpha_5 \left( \frac{1}{8} a^3 \omega_L - \frac{1}{8} a^3 \omega_L \cos(4\omega_L t + 4\beta) \right) \right\} \end{aligned} \quad (39)$$

By substituting (39) into (36), we obtain:

$$\begin{aligned}
 a\dot{\beta} = \frac{\varepsilon}{\omega_L} & \left\{ -\mu a \omega_L \sin(2\omega_L t + 2\beta) + \alpha_2 \left( \frac{3a^2}{4} \cos(\omega_L t + \beta) + \frac{a^2}{4} \cos(3\omega_L t + 3\beta) \right) \right. \\
 & + \alpha_3 \left( \frac{3a^3}{8} + \frac{a^3}{2} \cos(2\omega_L t + 2\beta) + \frac{a^3}{8} \cos(4\omega_L t + 4\beta) \right) \\
 & - \alpha_4 \left( \frac{a^2}{4} \omega_L \sin(\omega_L t + \beta) + \frac{a^2}{4} \omega_L \sin(3\omega_L t + 3\beta) \right) - \frac{F_1}{2_1} \cos((\omega_L - \omega)t + \beta) \\
 & \left. - \alpha_5 \left( \frac{a^3}{4} \omega_L \sin(2\omega_L t + 2\beta) + \frac{a^3}{8} \omega_L \sin(4\omega_L t + 4\beta) \right) - \frac{F_1}{2} \cos((\omega_L + \omega)t + \beta) \right\} \quad (40)
 \end{aligned}$$

In primary resonance,  $\omega \approx \omega_L$  and the expressions in  $\sin((\omega_L - \omega)t + \beta)$  and  $\cos((\omega_L - \omega)t + \beta)$  vary slowly with respect to time in Equations (39) and (40), respectively. We then have:

$$\begin{cases} \dot{a} = -\mu a \varepsilon - \alpha_5 \varepsilon \frac{a^3}{8} - \frac{\varepsilon F_1}{2\omega_L} \sin((\omega_L - \omega)t + \beta) \\ a\dot{\beta} = \alpha_3 \varepsilon \frac{3a^3}{8\omega_L} - \frac{\varepsilon F_1}{2\omega_L} \cos((\omega_L - \omega)t + \beta) \end{cases} \quad (41)$$

By setting  $\gamma = (\omega_L - \omega)t + \beta$  and  $\omega = \omega_L + \varepsilon\sigma$ , the system (41) becomes:

$$\begin{cases} \dot{a} = -\mu a \varepsilon - \alpha_5 \varepsilon \frac{a^3}{8} - \frac{\varepsilon F_1}{2\omega_L} \sin(\gamma) \\ a\dot{\gamma} + \varepsilon a \sigma = \alpha_3 \varepsilon \frac{3a^3}{8\omega_L} - \frac{\varepsilon F_1}{2\omega_L} \cos(\gamma) \end{cases} \quad (42)$$

Initially,  $a$  and  $\gamma$  oscillate, and as time increases, they become constants. Thus, for  $\dot{a} = 0$  and  $\dot{\gamma} = 0$ , we have:

$$\begin{cases} \frac{\mu}{\omega_L} + \frac{\alpha_5 a^2}{8\omega_L} = \frac{-F_1}{2a\omega_L^2} \sin(\gamma) \\ \sigma - 3 \frac{\alpha_3 a^2}{8\omega_L} = \frac{-F_1}{2a\omega_L^2} \cos(\gamma) \end{cases} \quad (43)$$

From system (43), the following equation is obtained:

$$\left[ \frac{\omega}{\omega_L} - \left( 1 + 3 \frac{\alpha_3 a^2}{8\omega_L^2} \right) \right]^2 + \left[ \frac{\mu}{\omega_L} + \frac{\alpha_5 a^2}{8\omega_L} \right]^2 = \left( \frac{F_1}{2a\omega_L^2} \right)^2 \quad (44)$$

### 3.2. Secondary Resonance

In the case of secondary resonance, Equation (31) can be rewritten by introducing the perturbation parameter in the following form:

$$\ddot{q} + \omega_L^2 q = -\varepsilon \left[ 2\mu\dot{q} + \alpha_2 q^2 + \alpha_3 q^3 + \alpha_4 q\dot{q} + \alpha_5 q^2 \dot{q} \right] + F_1 \cos(\omega t) \quad (45)$$

If  $\varepsilon = 0$  and using the principle of superposition, the general solution of Equation (45) is given by:

$$q = a \cos(\omega_L t + \beta) + 2\Lambda \cos(\omega t) \quad (46)$$

$$2\Lambda = \frac{F_1}{\omega_L^2 - \omega^2} \quad (47)$$

The derivative of (46) is given as follows:

$$\dot{q} = -a\omega_L \sin(\omega_L t + \beta) - 2\Lambda\omega \sin(\omega t) \quad (48)$$

If  $\varepsilon \neq 0$ , as in primary resonance, the derivative of (47), using the variation of constants, is given by:

$$\dot{q} = -a\omega_L \sin(\omega_L t + \beta) + \dot{a} \cos(\omega_L t + \beta) - a\dot{\beta} \sin(\omega_L t + \beta) - 2\Lambda\omega \sin(\omega t) \quad (49)$$

By comparing (48) and (49), we obtain:

$$\dot{a} \cos(\omega_L t + \beta) - a\dot{\beta} \sin(\omega_L t + \beta) = 0 \quad (50)$$

The derivative of (47) is given by:

$$\ddot{q} = -a\omega_L^2 \cos(\omega_L t + \beta) - \dot{a}\omega_L \sin(\omega_L t + \beta) - a\dot{\beta}\omega_L \cos(\omega_L t + \beta) - 2\Lambda\omega^2 \cos(\omega t) \quad (51)$$

By substituting (46), (48), and (30) into (45), we obtain:

$$\begin{aligned} & \dot{a}\omega_L \sin(\omega_L t + \beta) + a\dot{\beta} \cos(\omega_L t + \beta) = -2\mu\varepsilon [a\omega_L \sin(\omega_L t + \beta) + 2\Lambda\omega \sin(\omega t)] \\ & + \alpha_2\varepsilon [a \cos(\omega_L t + \beta) + 2\Lambda \cos(\omega t)]^2 + \alpha_3\varepsilon [a \cos(\omega_L t + \beta) + 2\Lambda \cos(\omega t)]^3 \\ & + \alpha_4\varepsilon [(a \cos(\omega_L t + \beta) + 2\Lambda \cos(\omega t))(-a\omega_L \sin(\omega_L t + \beta) - 2\Lambda\omega \sin(\omega t))] \\ & + \alpha_5\varepsilon [(a \cos(\omega_L t + \beta) + 2\Lambda \cos(\omega t))^2 (-a\omega_L \sin(\omega_L t + \beta) - 2\Lambda\omega \sin(\omega t))] \end{aligned} \quad (52)$$

Using (50) and (52), we obtain:

$$\begin{aligned} \dot{a} = & \frac{\varepsilon}{\omega_L} \left\{ -\mu a \omega_L (1 - \cos(2\omega_L t + 2\beta)) + 2\mu\Lambda\omega \cos((\omega_L + \omega)t + \beta) - 2\mu\Lambda\omega \cos((\omega_L - \omega)t + \beta) \right\} \\ & + \frac{\alpha_2\varepsilon}{\omega_L} \left\{ \left( 2\Lambda^2 + \frac{a^2}{4} \right) \sin(\omega_L t + \beta) + \frac{a^2}{4} \sin(3\omega_L t + 3\beta) + \Lambda^2 \sin((\omega_L + 2\omega)t + \beta) \right. \\ & \left. + \Lambda^2 \sin((\omega_L - 2\omega)t + \beta) + a\Lambda \sin((2\omega_L + \omega)t + 2\beta) + a\Lambda \sin((2\omega_L - \omega)t + 2\beta) \right\} \\ & + \frac{\alpha_3\varepsilon}{\omega_L} \left\{ \left( 3a\Lambda^2 + \frac{a^3}{4} \right) \sin(2\omega_L t + 2\beta) + \frac{a^3}{8} \sin(4\omega_L t + 4\beta) \right. \\ & + \left( 3\Lambda^3 + \frac{3a^2\Lambda}{4} \right) \sin((\omega_L + \omega)t + \beta) + \left( 3\Lambda^3 + \frac{3a^2\Lambda}{4} \right) \sin((\omega_L - \omega)t + \beta) \\ & + \frac{3a^2\Lambda}{2} \sin((2\omega_L + 2\omega)t + 2\beta) + \frac{3a^2\Lambda}{2} \sin((2\omega_L - 2\omega)t + 2\beta) + \Lambda^3 \sin((\omega_L + 3\omega)t + \beta) \\ & \left. + \frac{3a^2\Lambda}{4} \sin((3\omega_L + \omega)t + 3\beta) + \frac{3a^2\Lambda}{4} \sin((3\omega_L - \omega)t + 3\beta) + \Lambda^3 \sin((\omega_L - 3\omega)t + \beta) \right\} \\ & + \frac{\alpha_4\varepsilon}{\omega_L} \left\{ -\frac{a^2\omega_L}{4} \cos(\omega_L t + \beta) + \frac{a^2\omega_L}{4} \cos(3\omega_L t + 3\beta) - \Lambda a \omega_L \cos(\omega t) \right. \\ & \left. + \frac{a\Lambda}{2} (\omega_L + \omega) \cos((2\omega_L + \omega)t + 2\beta) + \frac{a\Lambda}{2} (\omega_L - \omega) \cos((2\omega_L - \omega)t + 2\beta) \right\} \end{aligned}$$

$$\begin{aligned}
 & + \omega\Lambda^2 \cos((2\omega_L + \omega)t + 2\beta) - \omega\Lambda^2 \cos((2\omega_L - \omega)t + 2\beta) \Big\} \\
 & + \frac{\alpha_5 \varepsilon}{\omega_L} \left\{ - \left( a\omega_L \Lambda^2 + \frac{a^3 \omega_L}{8} \right) - a\omega_L \Lambda^2 (\cos(2\omega t) + \cos(2\omega_L t + 2\beta)) + \frac{a^3 \omega_L}{8} \cos(4\omega_L t + 4\beta) \right. \\
 & + \left( \omega\Lambda^3 - \frac{a^2 \omega_L \Lambda}{2} + \frac{a^2 \omega \Lambda}{4} \right) \cos((\omega_L + \omega)t + \beta) - \left( \omega\Lambda^3 + \frac{a^2 \omega_L \Lambda}{2} + \frac{a^2 \omega \Lambda}{4} \right) \cos((\omega_L - \omega)t + \beta) \\
 & + \left( \frac{a^2 \omega_L \Lambda}{2} + \frac{a^2 \omega \Lambda}{4} \right) \cos((3\omega_L + \omega)t + 3\beta) + \left( \frac{a^2 \omega_L \Lambda}{2} - \frac{a^2 \omega \Lambda}{4} \right) \cos((3\omega_L - \omega)t + 3\beta) \\
 & + \left( a\omega\Lambda^2 + \frac{a\omega_L \Lambda^2}{2} \right) \cos((2\omega_L + 2\omega)t + 2\beta) + \left( -a\omega\Lambda^2 + \frac{a\omega_L \Lambda^2}{2} \right) \cos((2\omega_L - 2\omega)t + 2\beta) \\
 & \left. + \omega\Lambda^3 \cos((\omega_L + 3\omega)t + \beta) - \omega\Lambda^3 \cos((\omega_L - 3\omega)t + \beta) \right\} \tag{53}
 \end{aligned}$$

By substituting (53) into (50), we obtain:

$$\begin{aligned}
 a\dot{\beta} = & \frac{\varepsilon}{\omega_L} \left\{ -\mu a \omega_L (1 - \sin(2\omega_L t + 2\beta)) - 2\mu \Lambda \omega \sin((\omega_L + \omega)t + \beta) + 2\mu \Lambda \omega \sin((\omega_L - \omega)t + \beta) \right\} \\
 & + \frac{\alpha_2 \varepsilon}{\omega_L} \left\{ \left( 2\Lambda^2 + \frac{3a^2}{4} \right) \cos(\omega_L t + \beta) + 2a\Lambda \cos(\omega t) + \frac{a^2}{4} \cos(3\omega_L t + 3\beta) + \Lambda^2 \cos((\omega_L + 2\omega)t + \beta) \right. \\
 & \left. + \Lambda^2 \cos((\omega_L - 2\omega)t + \beta) + a\Lambda \cos((2\omega_L + \omega)t + 2\beta) + a\Lambda \cos((2\omega_L - \omega)t + 2\beta) \right\} \\
 & + \frac{\alpha_3 \varepsilon}{\omega_L} \left\{ \left( 3a\Lambda^2 + \frac{3a^3}{8} \right) + \left( 3a\Lambda^2 + \frac{a^3}{2} \right) \cos(2\omega_L t + 2\beta) + \frac{a^3}{8} \cos(4\omega_L t + 4\beta) + 3a\Lambda^2 \cos(2\omega t) \right. \\
 & + \left( 3\Lambda^3 + \frac{9a^2 \Lambda}{4} \right) \cos((\omega_L + \omega)t + \beta) + \left( 3\Lambda^3 + \frac{9a^2 \Lambda}{4} \right) \cos((\omega_L - \omega)t + \beta) \\
 & + \frac{3a^2 \Lambda}{2} \cos((2\omega_L + 2\omega)t + 2\beta) + \frac{3a^2 \Lambda}{2} \cos((2\omega_L - 2\omega)t + 2\beta) + \Lambda^3 \cos((\omega_L + 3\omega)t + \beta) \\
 & \left. + \frac{3a^2 \Lambda}{4} \cos((3\omega_L + \omega)t + 3\beta) + \frac{3a^2 \Lambda}{4} \cos((3\omega_L - \omega)t + 3\beta) + \Lambda^3 \cos((\omega_L - 3\omega)t + \beta) \right\} \\
 & + \frac{\alpha_4 \varepsilon}{\omega_L} \left\{ -\frac{a^2 \omega_L}{4} \sin(\omega_L t + \beta) - \frac{a^2 \omega_L}{4} \sin(3\omega_L t + 3\beta) - \Lambda a \omega_L \sin(\omega t) \right. \\
 & - \frac{a\Lambda}{2} (\omega_L + \omega) \sin((2\omega_L + \omega)t + 2\beta) - \frac{a\Lambda}{2} (\omega_L - \omega) \sin((2\omega_L - \omega)t + 2\beta) \\
 & \left. - \omega\Lambda^2 \sin((2\omega_L + \omega)t + 2\beta) + \omega\Lambda^2 \sin((2\omega_L - \omega)t + 2\beta) \right\} \\
 & + \frac{\alpha_5 \varepsilon}{\omega_L} \left\{ - \left( a\omega_L \Lambda^2 + \frac{a^3 \omega_L}{4} \right) \sin(2\omega_L t + 2\beta) - 2a\omega\Lambda^2 \sin(2\omega t) - \frac{a^3 \omega_L}{8} \sin(4\omega_L t + 4\beta) \right. \\
 & - \left( \omega\Lambda^3 + \frac{a^2 \omega_L \Lambda}{2} + \frac{3a^2 \omega \Lambda}{4} \right) \sin((\omega_L + \omega)t + \beta) + \left( \omega\Lambda^3 - \frac{a^2 \omega_L \Lambda}{2} + \frac{3a^2 \omega \Lambda}{4} \right) \sin((\omega_L - \omega)t + \beta) \\
 & - \left( \frac{a^2 \omega_L \Lambda}{2} + \frac{a^2 \omega \Lambda}{4} \right) \sin((3\omega_L + \omega)t + 3\beta) - \left( \frac{a^2 \omega_L \Lambda}{2} - \frac{a^2 \omega \Lambda}{4} \right) \sin((3\omega_L - \omega)t + 3\beta) \\
 & - \left( a\omega\Lambda^2 + \frac{a\omega_L \Lambda^2}{2} \right) \sin((2\omega_L + 2\omega)t + 2\beta) - \left( -a\omega\Lambda^2 + \frac{a\omega_L \Lambda^2}{2} \right) \sin((2\omega_L - 2\omega)t + 2\beta) \\
 & \left. - \omega\Lambda^3 \sin((\omega_L + 3\omega)t + \beta) + \omega\Lambda^3 \sin((\omega_L - 3\omega)t + \beta) \right\} \tag{54}
 \end{aligned}$$

In secondary resonance, there are two cases: superharmonic resonance and subharmonic resonance.

**3.2.1. Superharmonic Resonance**

In this case, we have  $\omega \approx \frac{1}{3}\omega_L$  et  $\omega \approx \frac{1}{2}\omega_L$ .

**First case:**  $\omega \approx \frac{1}{3}\omega_L, \Lambda = \frac{9F_1}{\omega_L^2}$

The expressions for  $\sin((\omega_L - 3\omega)t + \beta)$  and  $\cos((\omega_L - 3\omega)t + \beta)$  vary slowly with time. Equations (53) and (54) then become:

$$\begin{cases} \dot{a} + \mu a \varepsilon + \frac{\alpha_5 \varepsilon a^3}{8} + \alpha_5 \varepsilon a \Lambda^2 = \frac{\alpha_3 \varepsilon \Lambda^3}{\omega_L} \sin((\omega_L - 3\omega)t + \beta) - \frac{\alpha_5 \varepsilon \Lambda^3}{3\omega_L} \cos((\omega_L - 3\omega)t + \beta) \\ a \dot{\beta} - \frac{3\alpha_3 \varepsilon a^3}{8\omega_L} - \frac{3\alpha_3 \varepsilon a \Lambda^2}{\omega_L} = \frac{\alpha_3 \varepsilon \Lambda^3}{\omega_L} \cos((\omega_L - 3\omega)t + \beta) + \frac{\alpha_5 \varepsilon \Lambda^3}{3\omega_L} \sin((\omega_L - 3\omega)t + \beta) \end{cases} \quad (55)$$

By setting  $\gamma = (\omega_L - 3\omega)t + \beta$  and  $3\omega = \omega_L + \varepsilon\sigma$ , the system (55) becomes:

$$\begin{cases} \dot{a} + \mu a \varepsilon + \frac{\alpha_5 \varepsilon a^3}{8} + \alpha_5 \varepsilon a \Lambda^2 = \frac{\alpha_3 \varepsilon \Lambda^3}{\omega_L} \sin(\gamma) - \frac{\alpha_5 \varepsilon \Lambda^3}{3\omega_L} \cos(\gamma) \\ a \dot{\gamma} + \varepsilon a \sigma - \frac{3\alpha_3 \varepsilon a^3}{8\omega_L} - \frac{3\alpha_3 \varepsilon a \Lambda^2}{\omega_L} = \frac{\alpha_3 \varepsilon \Lambda^3}{\omega_L} \cos(\gamma) + \frac{\alpha_5 \varepsilon \Lambda^3}{3\omega_L} \sin(\gamma) \end{cases} \quad (56)$$

If the system tends toward a steady state,  $\dot{a} = 0$  and  $\dot{\gamma} = 0$ , we have:

$$\begin{cases} \mu + \frac{\alpha_5 a^2}{8} + \alpha_5 \Lambda^2 = \frac{\alpha_3 \Lambda^3}{a \omega_L} \sin(\gamma) - \frac{\alpha_5 \Lambda^3}{3a \omega_L} \cos(\gamma) \\ \sigma - \frac{3\alpha_3 a^2}{8\omega_L} - \frac{3\alpha_3 \Lambda^2}{\omega_L} = \frac{\alpha_3 \Lambda^3}{a \omega_L} \cos(\gamma) + \frac{\alpha_5 \Lambda^3}{3a \omega_L} \sin(\gamma) \end{cases} \quad (57)$$

From system (57), we obtain:

$$\left[ \frac{\omega}{\omega_L} - \left( \frac{1}{3} + \frac{\alpha_3 a^2}{8\omega_L^2} + \frac{\alpha_3 \Lambda^2}{\omega_L^2} \right) \right]^2 + \left[ \frac{\mu}{3\omega_L} + \frac{\alpha_5 a^2}{24\omega_L} + \frac{\alpha_5 \Lambda^2}{3\omega_L} \right]^2 = \left( \frac{\alpha_3 \Lambda^3}{3a \omega_L^2} \right)^2 + \left( \frac{\alpha_5 \Lambda^3}{9a \omega_L} \right)^2 \quad (58)$$

**Second case:**  $\omega \approx \frac{1}{2}\omega_L, \Lambda = \frac{2F_1}{3\omega_L^2}$

The expressions for  $\sin((\omega_L - 2\omega)t + \beta)$  and  $\cos((\omega_L - 2\omega)t + \beta)$  vary slowly with time. Equations (53) and (54) then become:

$$\begin{cases} \dot{a} + \mu a \varepsilon + \frac{\alpha_5 \varepsilon a^3}{8} + \alpha_5 \varepsilon a \Lambda^2 = \frac{\alpha_2 \varepsilon \Lambda^2}{\omega_L} \sin((\omega_L - 2\omega)t + \beta) - \frac{\alpha_4 \varepsilon \Lambda^2}{2\omega_L} \cos((\omega_L - 2\omega)t + \beta) \\ a \dot{\beta} - \frac{3\alpha_3 \varepsilon a^3}{8\omega_L} - \frac{3\alpha_3 \varepsilon a \Lambda^2}{\omega_L} = \frac{\alpha_2 \varepsilon \Lambda^2}{\omega_L} \cos((\omega_L - 2\omega)t + \beta) + \frac{\alpha_4 \varepsilon \Lambda^2}{2\omega_L} \sin((\omega_L - 2\omega)t + \beta) \end{cases} \quad (59)$$

By setting  $\gamma = (\omega_L - 2\omega)t + \beta$  et  $2\omega = \omega_L + \varepsilon\sigma$ , the system (59) becomes:





- C-S: clamped-simply supported beam;
- C-C: clamped-clamped beam.

The geometric and material properties are presented in **Table 1**. The numerical values corresponding to the different boundary conditions are given in **Table 2**.

**Table 1.** Geometric and material properties of the beam [12].

	Elastic layer	Piezoelectric layer
$L$ : Length (m)	1	1
$H$ : Width (m)	$H = 5h$	$H = 5h$
$h$ : Total thickness ( $h = 0.01$ )	$h_c = 5h \div 6$	$h_a = h_s = H \div 12$
Young's modulus (Pa)	$E_c = 6.9 \times 10^{10}$	---
Density ( $\text{Kg}\cdot\text{m}^{-3}$ )	$\rho_c = 2766$	$\rho_s = 7500$
$c_{11}^*$ (Pa)	---	$6.98 \times 10^{10}$
$e_{11}^*$ ( $\text{C}\cdot\text{m}^{-2}$ )	---	-23.2
$\epsilon_{11}^*$ ( $\text{F}\cdot\text{m}^{-1}$ )	---	$1.73 \times 10^{-8}$

**Table 2.** Coefficient values corresponding to the three boundary conditions [12].

	S-S	C-S	C-C
$\mu_1$	$1.4924 \times 10^3$	$3.6420 \times 10^3$	$7.6689 \times 10^3$
$\mu_2$	$-3.4548 \times 10^5$	$-2.9565 \times 10^5$	0
$\mu_3$	$-1.7760 \times 10^7$	$-2.3151 \times 10^7$	$-2.1882 \times 10^7$
$\mu_4$	$-4.8367 \times 10^5$	$-4.3922 \times 10^5$	0
$\mu_5$	$-3.5520 \times 10^7$	$-4.6302 \times 10^7$	$-4.3765 \times 10^7$
$C_1$	$1.7333 \times 10^4$	$4.2300 \times 10^4$	$8.9070 \times 10^4$
$C_2$	$1.5075 \times 10^7$	$0.18569 \times 10^7$	0
$C_3$	$4.9133 \times 10^8$	$6.4047 \times 10^8$	$6.0537 \times 10^8$

Using the feedback control parameters, the  $\mu_i$  and  $C_i$  coefficients in **Table 2** are given by:

$$2\mu = \mu_1 G_d; \quad \alpha_2 = C_2 + \mu_2 G_p; \quad \alpha_3 = C_3 + \mu_3 G_p; \\ \omega_L^2 = C_1 + \mu_1 G_p; \quad \alpha_4 = \mu_4 G_d; \quad \alpha_5 = \mu_5 G_d.$$

$$C_1 = \frac{1}{M} \left( E_c I_c + 2c_{11}^* (I_s + S_s z_s^2) + \frac{(ES)_{pe}}{S_s} (2I_s + S_s z_s^2) \right) \int_0^L \varphi_{xxxx}(x) \varphi(x) dx;$$

$$C_2 = \frac{(ES)_{pe} z_s}{M} \left( \frac{1}{L} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L \varphi_{xx}(x) dx - \int_0^L \left\{ (\varphi_{xx}(x))^2 + \varphi_x(x) \varphi_{xxx}(x) \right\} \varphi_x(x) dx \right);$$

$$\begin{aligned}
C_3 &= -\frac{E_c I_c + 2c_{11}^* S_s + (ES)_{pe}}{2ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L (\varphi_x(x))^2 dx; \\
\mu_1 &= \frac{(ES)_{pe} z_s^2}{M} \int_0^L \varphi_{xxxx}(x) \varphi(x) dx; \\
\mu_2 &= -\frac{(ES)_{pe} z_s}{M} \left( \frac{1}{L} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L \varphi_{xx}(x) dx + \int_0^L \left\{ (\varphi_{xx}(x))^2 + \varphi_x(x) \varphi_{xxx}(x) \right\} \varphi_x(x) dx \right); \\
\mu_3 &= \frac{(ES)_{pe}}{2ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L (\varphi_x(x))^2 dx; \\
\mu_4 &= -\frac{(ES)_{pe} z_s}{M} \left( \frac{1}{L} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L \varphi_{xx}(x) dx + 2 \int_0^L \left\{ (\varphi_{xx}(x))^2 + \varphi_x(x) \varphi_{xxx}(x) \right\} \varphi_x(x) dx \right); \\
\mu_5 &= \frac{(ES)_{pe}}{ML} \int_0^L \varphi_{xx}(x) \varphi(x) dx \int_0^L (\varphi_x(x))^2 dx; \\
F_1 &= \frac{1}{M} \int_0^L f(x) \varphi(x) dx; \quad M = (\rho S)_* \int_0^L (\varphi(x))^2 dx; \quad (ES)_{pe} = S_s \frac{(\epsilon_{31}^*)^2}{\epsilon_{33}^*}.
\end{aligned}$$

## 4.2. Primary Resonance

In all the presented figures, the frequency is normalized with respect to the natural frequency of the studied beam, and the beam is uniformly excited.

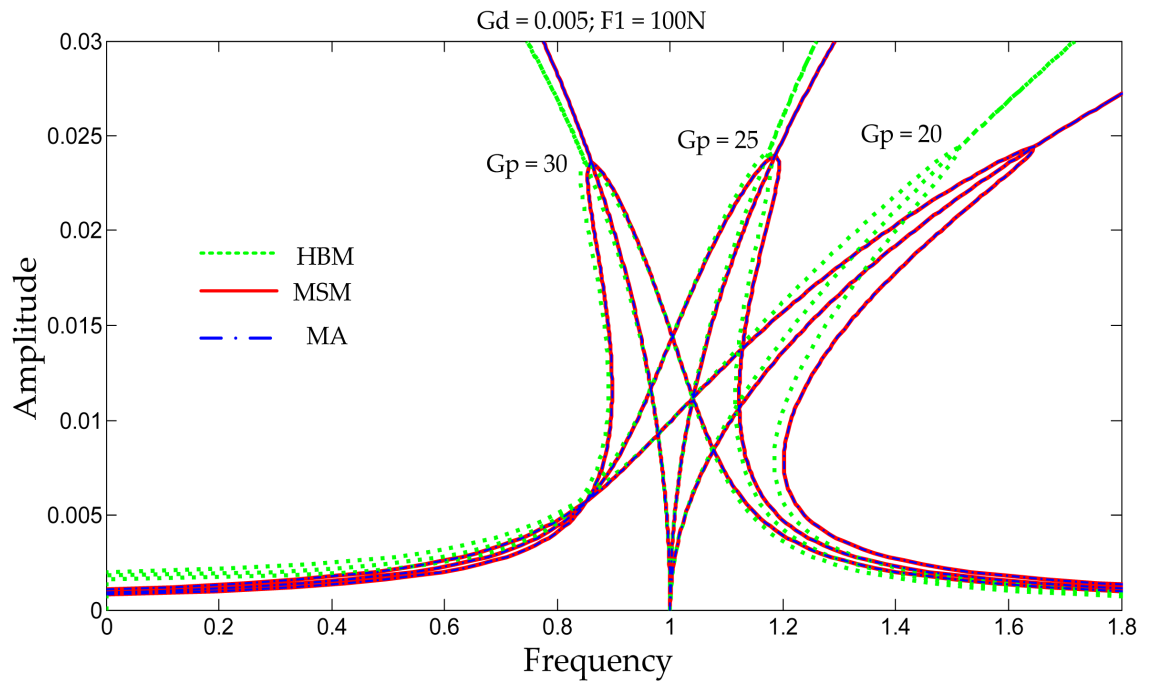
**Figure 2** compares three analytical methods: the method of averaging (MA), used in this work, the harmonic balance method (HBM) [16]-[20] [29], and the multiple scales method (MSM) [12] [30], which have been used in the literature. The curves obtained using the method of averaging and the multiple scales method overlap, as shown in the figure, whereas a slight deviation is observed with those obtained by the harmonic balance method. This is likely due to the lower accuracy of the latter method. The method of averaging can thus be considered a reliable technique for vibration control in structures with good accuracy.

**Figure 3** and **Figure 4** illustrate typical nonlinear phenomena. When  $G_p = 20$ , the stiffness increases, and the system is said to be hardening, which results in a frequency response curve leaning toward higher frequencies. When  $G_p = 35$ , the stiffness decreases, and the system is said to be softening, leading to a frequency response curve inclined toward lower frequencies. In both cases, multiple solutions may exist for the same excitation frequency. This gives rise to jump phenomena, depending on the direction of the frequency sweep.

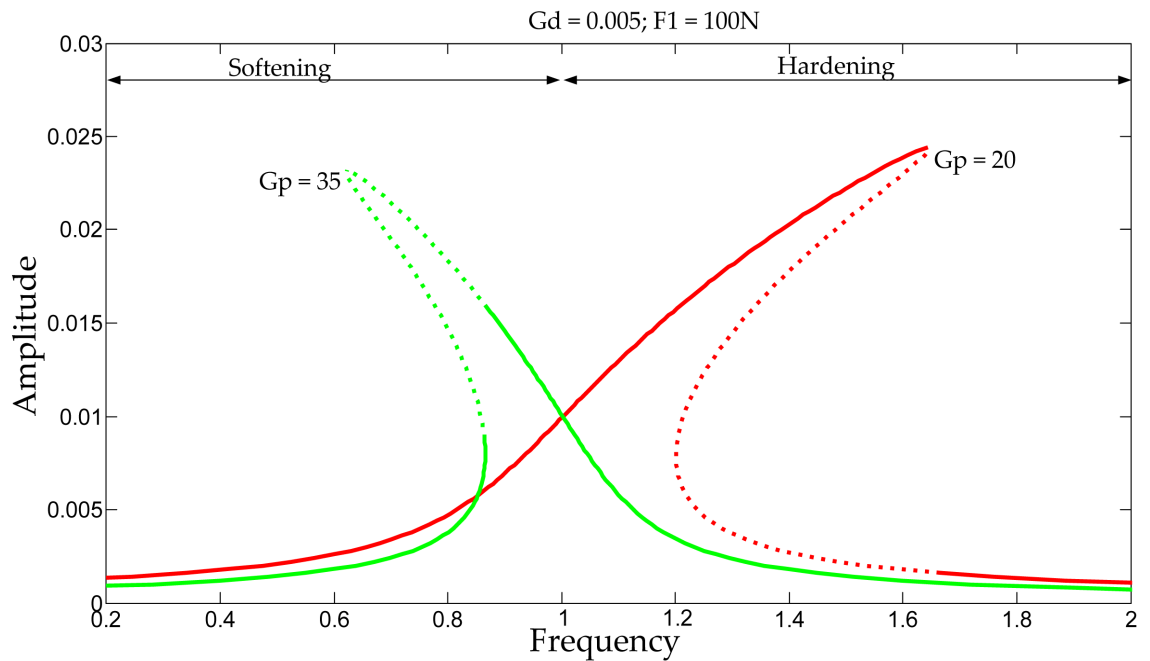
**Figure 5** presents the effects of boundary conditions on the amplitude-frequency response of the structure. The S-S beam is highly sensitive to changes in the  $G_p$  parameter compared to the C-S beam, and more sensitive than the C-C beam. The effects of geometric nonlinearities are significantly reduced when the value of this parameter is increased.

**Figure 6** shows the free responses, i.e., when the external force is zero. However, depending on the initial shape of the structure at rest, some behaviors can be softening, while others are hardening. This behavior may depend on the static stiffness of the structure around its equilibrium point and the presence of nonlinearities.

In **Figure 7**, the proportional control gain  $G_p$  is fixed at 20 while the velocity control gain  $G_d$  is varied. As  $G_d$  increases, the vibration amplitudes decrease, and the system remains in the hardening regime. To observe the opposite effect, i.e., the softening behavior, one simply needs to increase the value of  $G_p$ , as shown in **Figure 8**. The hardening-softening transition is independent of  $G_{db}$  since increasing or decreasing its value always results in a hardening behavior.



**Figure 2.** Comparison of the three analytical methods on the amplitude-frequency responses of the S-S beam.



**Figure 3.** Jumps in the increasing direction of frequencies.

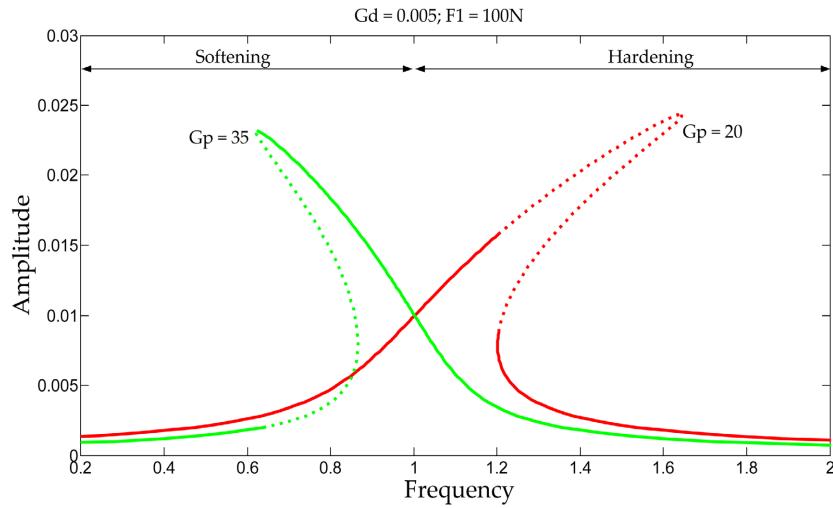


Figure 4. Jumps in the decreasing direction of frequencies.

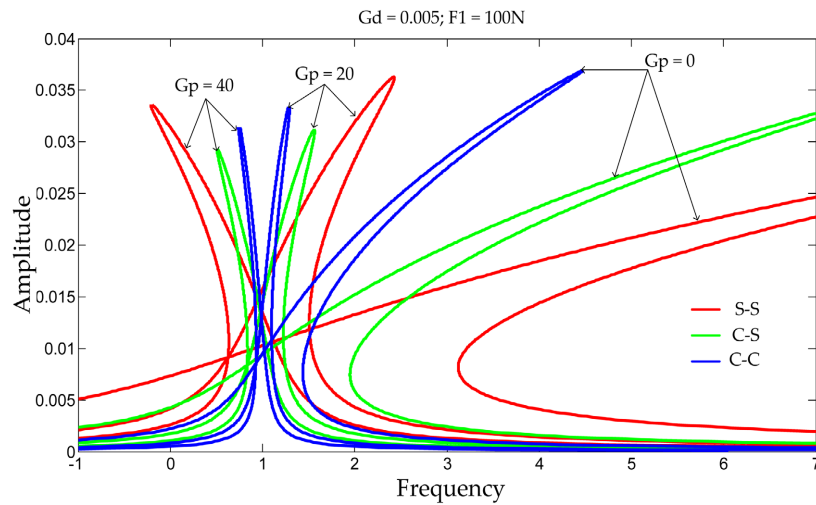


Figure 5. Amplitude-frequency responses for the three boundary conditions.

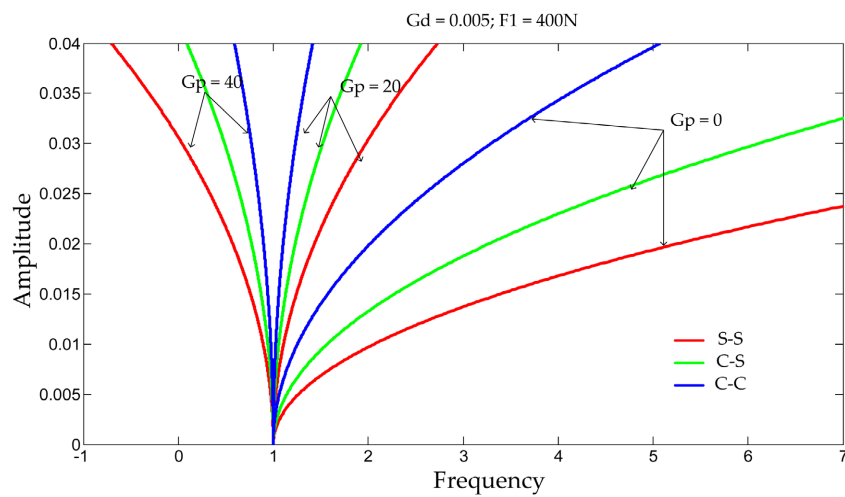


Figure 6. Free nonlinear amplitude-frequency responses for the three boundary conditions.

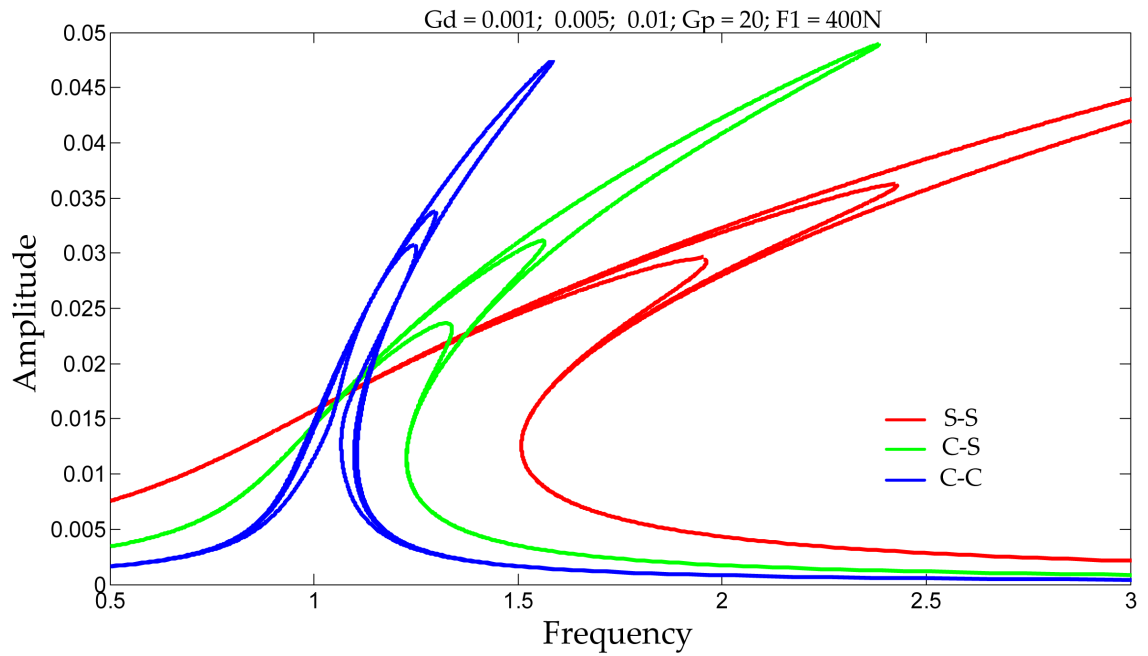


Figure 7. Amplitude-frequency responses of the three boundary conditions.

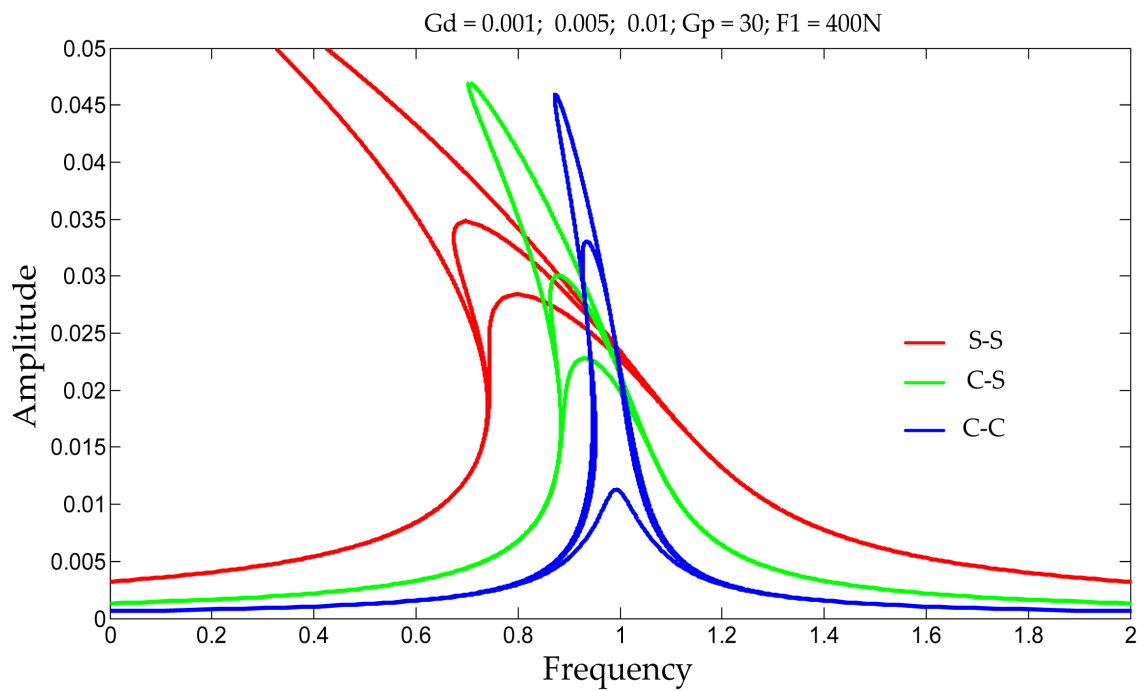


Figure 8. Amplitude-frequency responses of the three boundary conditions.

### 4.3. Secondary Resonance

#### 4.3.1. Superharmonic Resonance

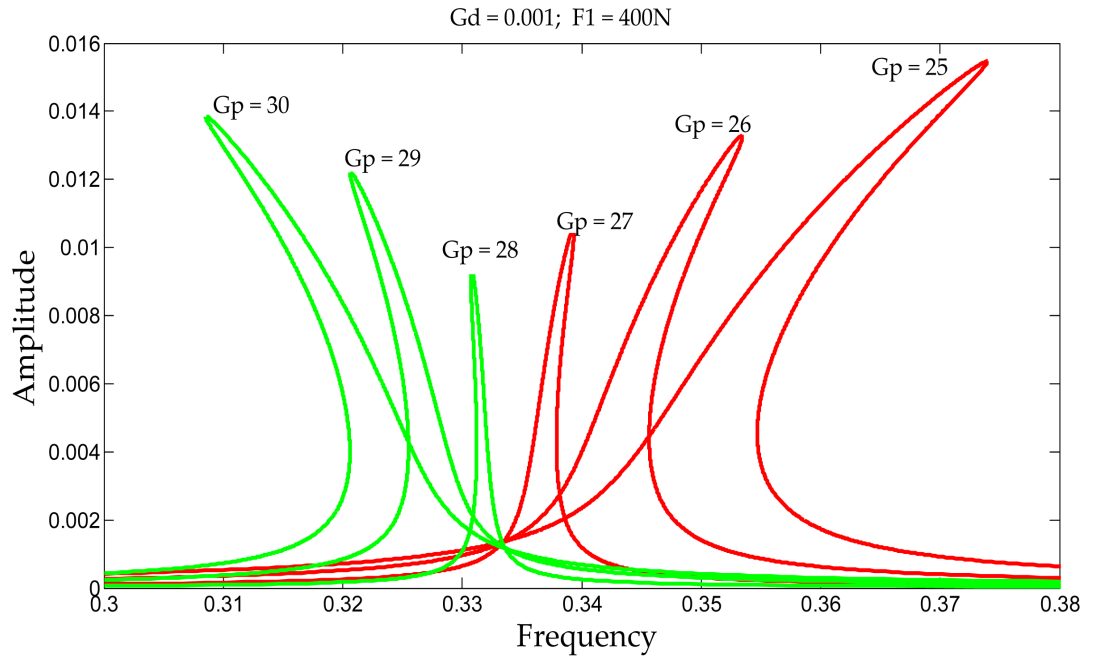
Figure 9 and Figure 10 show the effects of nonlinearities on beam S-S.

For  $\omega \approx 1/3 \omega_L$ , on the stiffening side, the amplitudes decrease as  $G_p$  increases; however, on the softening side, the amplitudes increase with increasing  $G_p$ .

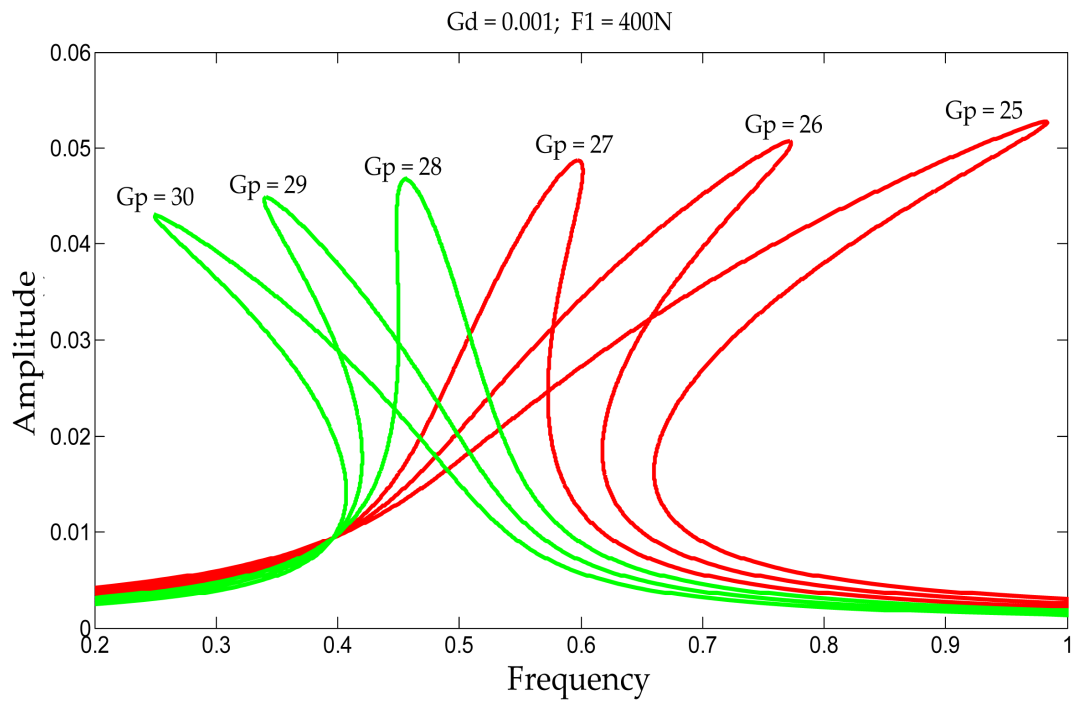
For  $\omega \approx 1/2 \omega_L$ , the vibration amplitudes decrease on both sides as  $G_p$  in-

creases. In this particular case, the transition can be controlled by the parameter  $G_p$  and the resonance shifts.

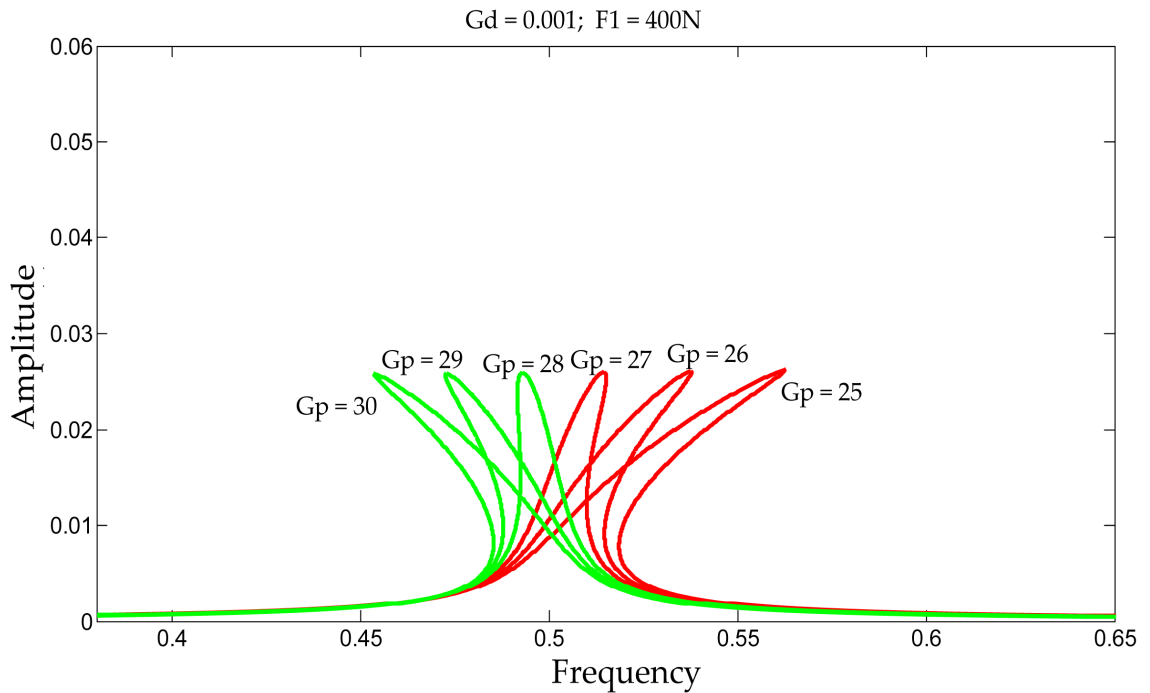
For the C-S beam in **Figure 11**, nonlinear behaviors occur around the structure's natural mode. In **Figure 12**, the excitation effect is negligible on the C-C beam. These behaviors result from the presence of clamping in the structure.



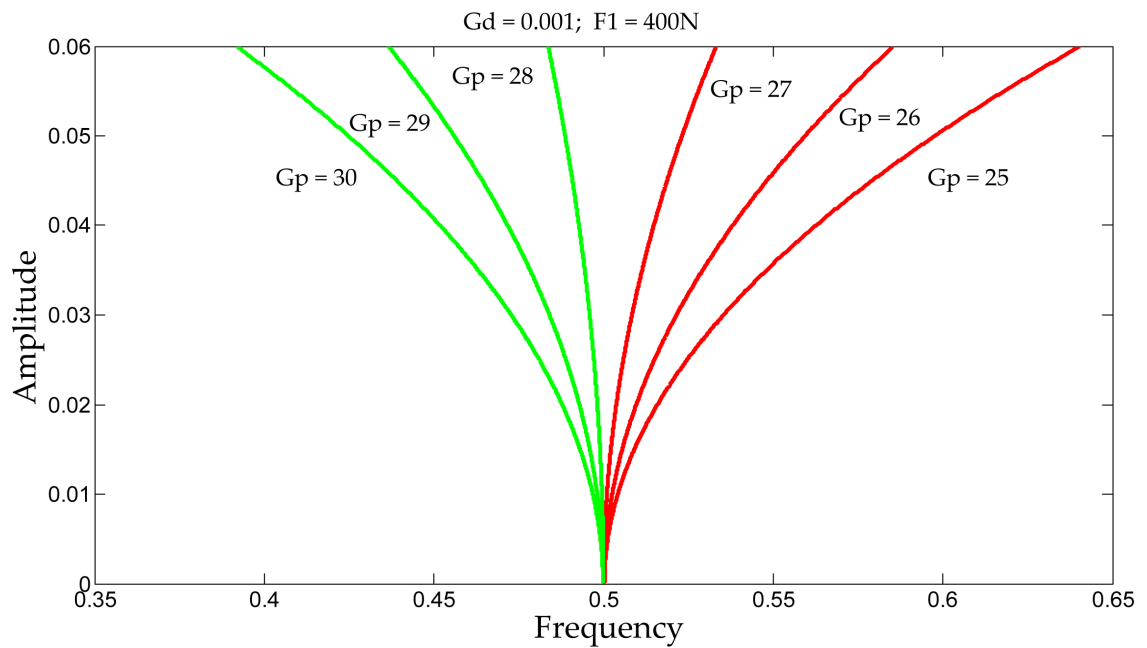
**Figure 9.** Amplitude-frequency superharmonic responses for  $\omega \approx 1/3 \omega_L$  of beam S-S.



**Figure 10.** Amplitude-frequency superharmonic responses for  $\omega \approx 1/2 \omega_L$  of beam S-S.



**Figure 11.** Amplitude-frequency superharmonic responses for  $\omega \approx 1/2\omega_L$  of beam C-S.

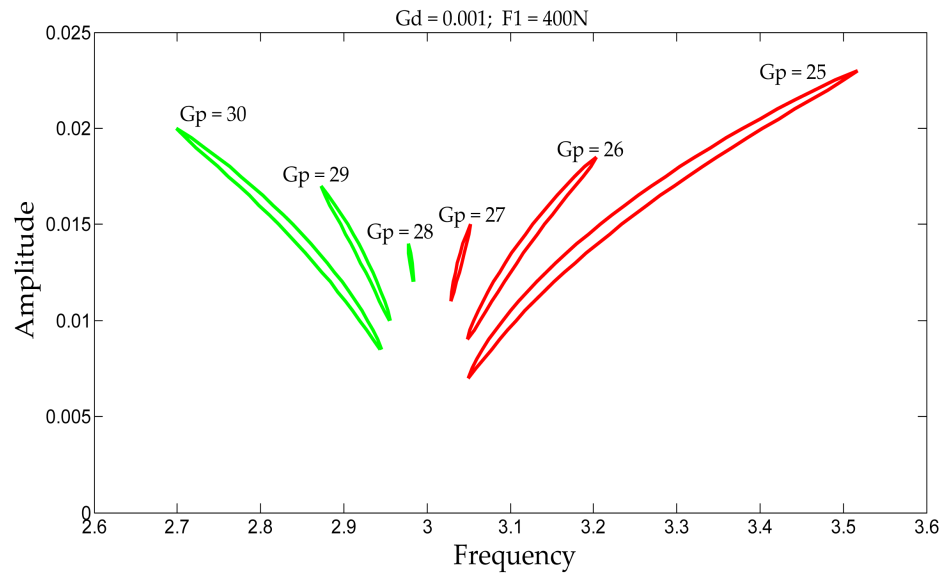


**Figure 12.** Super-harmonic amplitude-frequency response for  $\omega \approx 1/2\omega_L$  of the C-C beam.

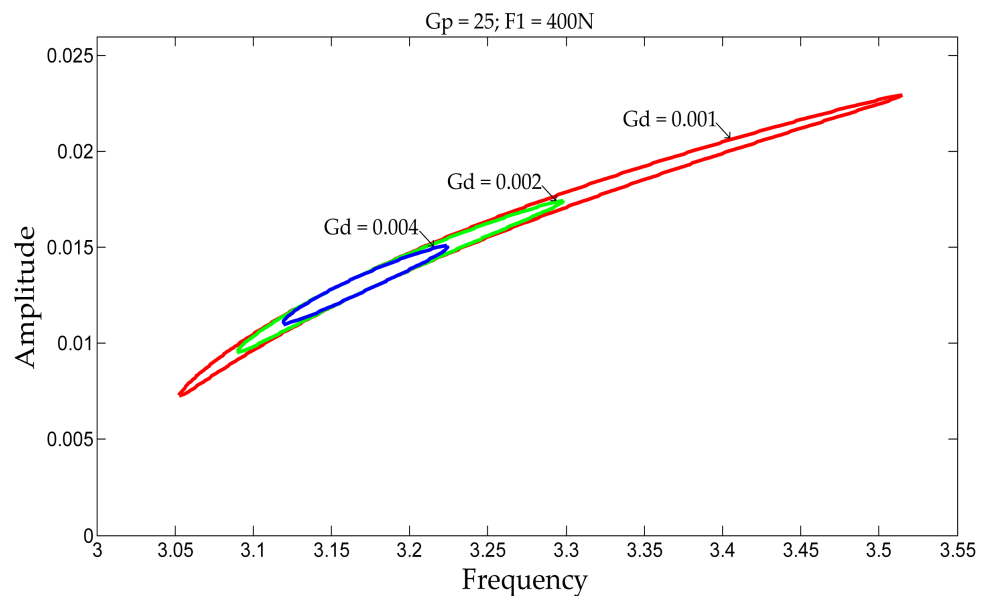
**4.3.2. Subharmonic Resonance**

In **Figure 13**, the behavior of beam S-S exhibits hysteresis. This suggests that the nonlinear relationship between amplitude and frequency is due to mechanical losses within the structure. These losses, having become a form of energy, dissipate within the structure; therefore, control gains help reduce these losses by decreasing the area of the amplitude-frequency hysteresis loop. On the stiffening

side for fixed  $G_{db}$  this area can be reduced by increasing the gain  $G_p$ , whereas on the softening side, the area is instead reduced by decreasing the gain. The same phenomenon is observed in **Figure 14** and **Figure 15**, with  $G_p$  fixed and  $G_d$  varied. For all values of  $G_{db}$  the beam exhibits stiffening behavior, and increasing its value reduces the hysteresis area. Thus, by minimizing this energy, the operational range of the beam can be optimized. These behaviors are significant for energy harvesting. Similarly, in **Figure 16** and **Figure 17**, second-order nonlinear effects are observed, and one can also control the vibration amplitudes as well as the stiffening-softening transition. As for **Figure 18**, it shows that the influence of excitation is negligible on beam C-C.



**Figure 13.** Subharmonic amplitude-frequency responses for  $\omega \approx 3\omega_L$  of the S-S beam.



**Figure 14.** Subharmonic amplitude-frequency responses for  $\omega \approx 3\omega_L$  of S-S beam.

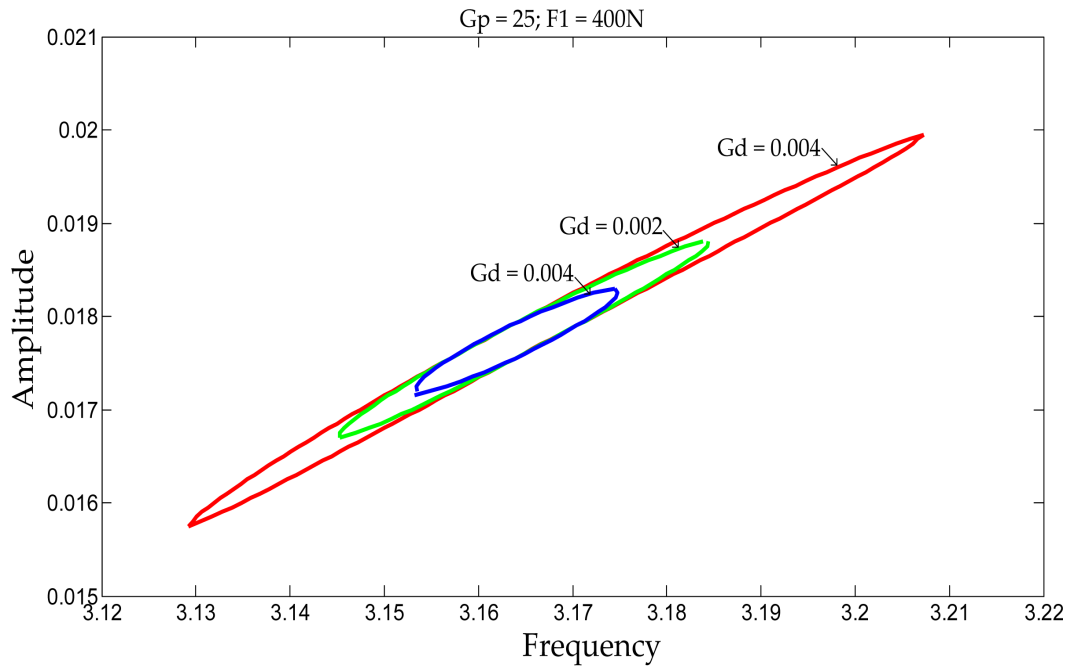


Figure 15. Subharmonic amplitude-frequency responses for  $\omega \approx 3\omega_L$  of the C-S beam.

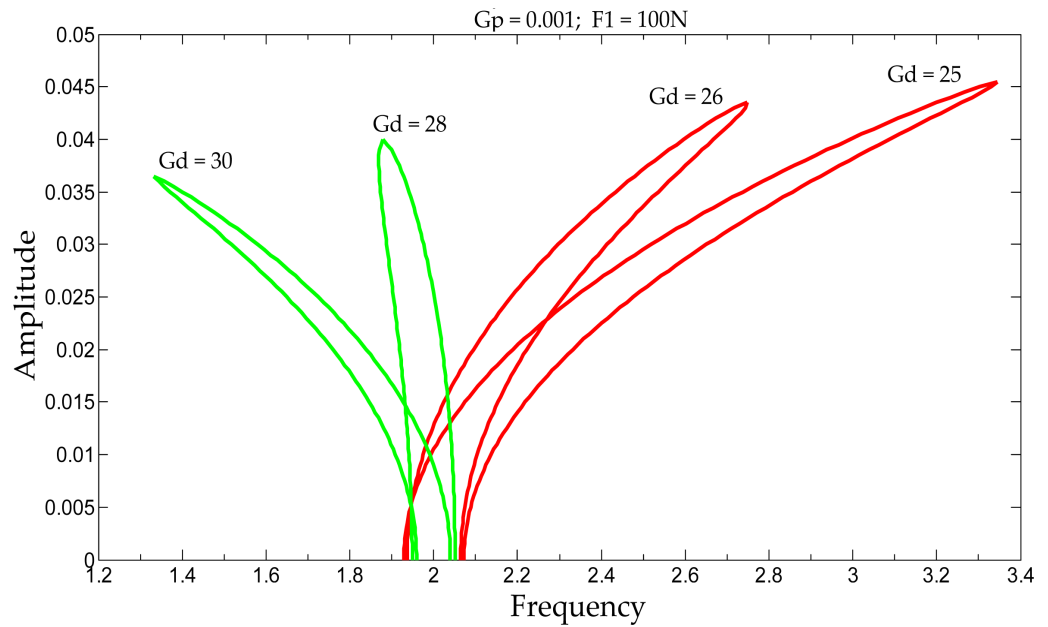
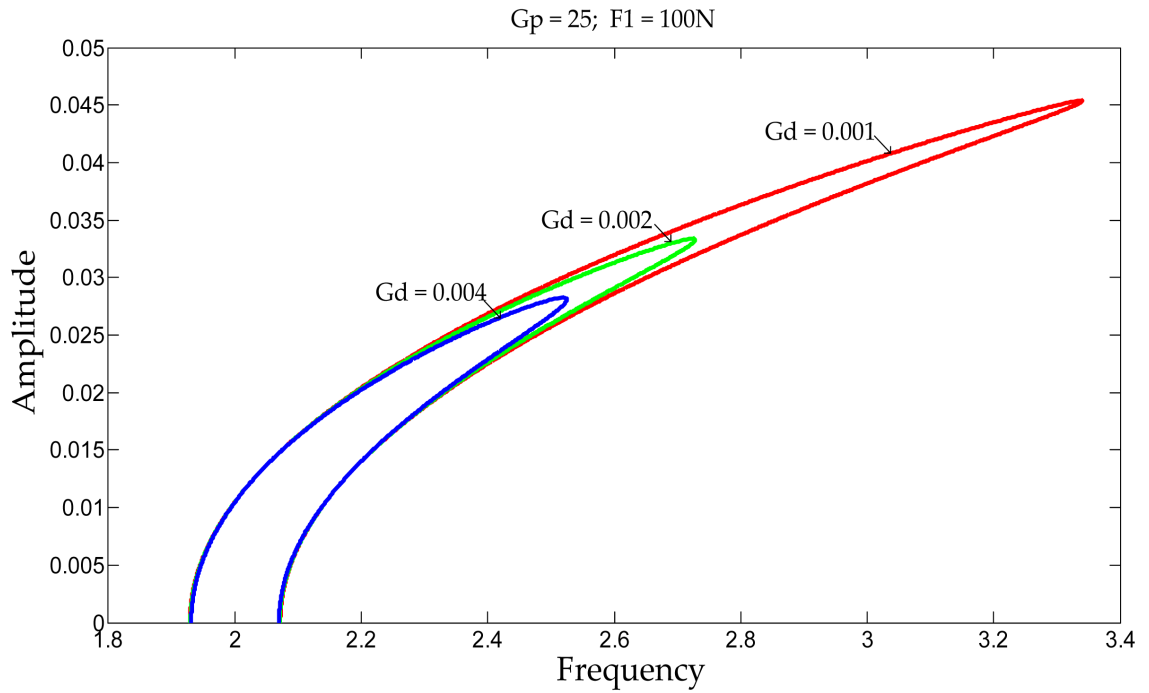
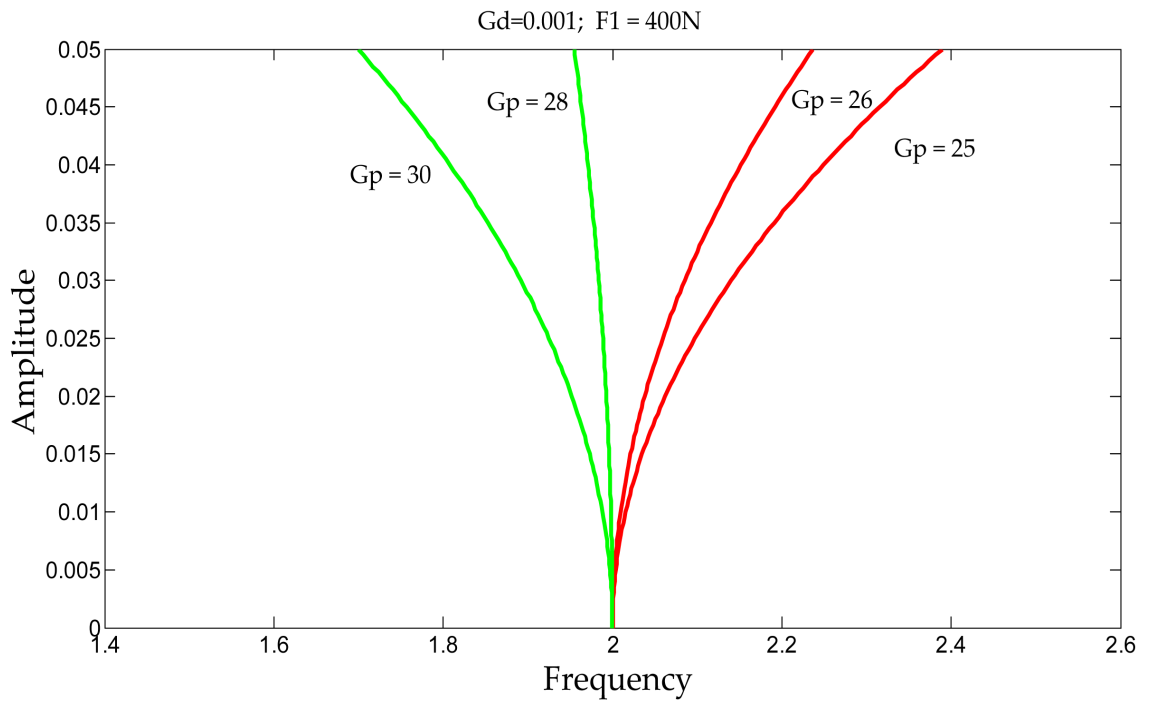


Figure 16. Subharmonic amplitude-frequency responses for  $\omega \approx 2\omega_L$  of the S-S beam.

The control of vibrational behavior can significantly influence the performance of energy harvesting devices. Maintaining the excitation amplitude and frequency close to the resonance point of a piezoelectric energy harvesting device allows for an increase in the harvested power. In this context, the proportional-derivative control applied in this study could optimize energy harvesting by adjusting the effective stiffness and damping, thereby modulating the vibrational response to maximize energy conversion efficiency.



**Figure 17.** Subharmonic amplitude-frequency responses for  $\omega \approx 2\omega_L$  of S-S beam.



**Figure 18.** Subharmonic amplitude-frequency responses for  $\omega \approx 2\omega_L$  of the C-C beam.

### 5. Conclusion

This study aimed to explore the method of averaging for active control of nonlinear vibrations in a sandwich beam. Based on the results obtained and supported by findings in the literature, this method has proven to be highly effective for active

vibration control and may also be very relevant for energy harvesting. Three boundary conditions were highlighted: simply supported beam, clamped-simply supported beam, and clamped-clamped beam. In general, the dynamic behavior of the beam can be controlled through control gains for all these boundary conditions using a retractable control law. The stiffening-softening transition can be managed via the control gain  $G_p$ , while amplitude reduction is achieved through the parameter  $G_d$ .

### Author Contributions

Barthelemy Zra Mha: Conceptualization, Methodology, Software, Writing, original draft. Guy Edgar Ntamack: Supervision, Methodology, Validation, Writing.

### Data Availability Statement

The data supporting the findings of this study were generated via numerical simulation and are available from the corresponding author upon reasonable request.

### Conflicts of Interest

The authors declare no conflicts of interest regarding the publication of this paper.

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